

CMT Components

8319 Ashridge Arnhim
Address 2
Sardinia, OH 45171

Truss: Residential

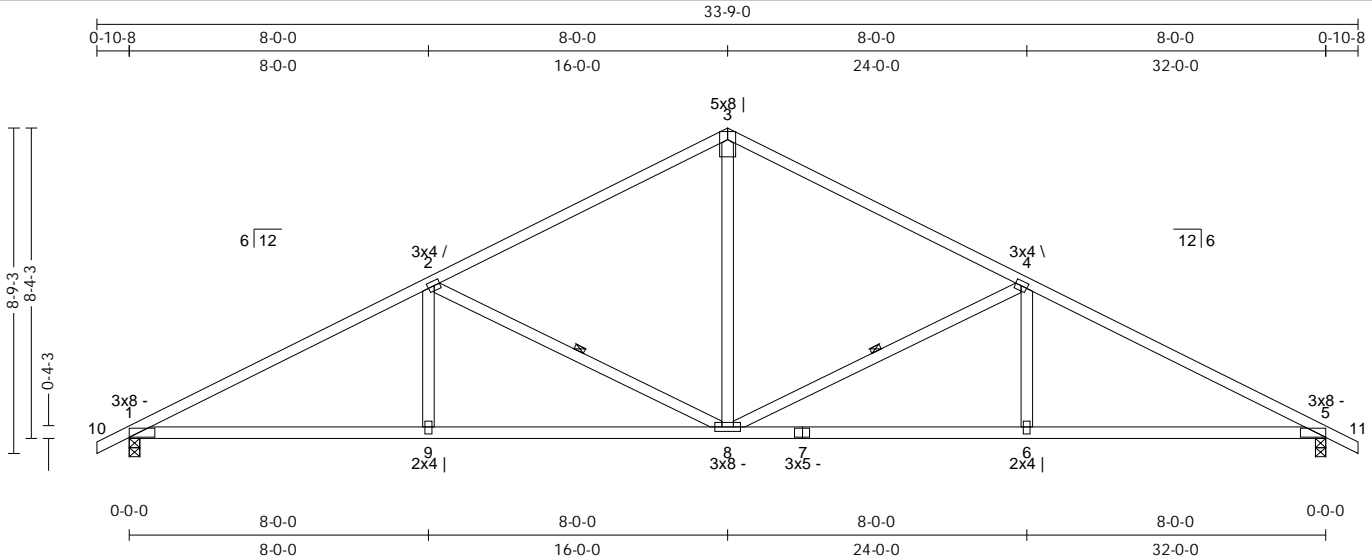
Job: D14aWalker

Designer:CN

Date: 04/14/26 07:51:35

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SPAN 32-0-0	PITCH 6/12	QTY 1	OHL 0-10-8	OHR 0-10-8	CANTL 0-0-0	CANTR 0-0-0	PLYS 1	SPACING 24in	WGT/PLY 143 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2015/	TC: 0.78 (3-4)	Vert TL: 0.26 in	L/999	(5-6)	L/360
GSL: 25	TP1 1-2014	BC: 0.64 (5-6)	Vert LL: 0.1 in	L/999	(5-6)	L/480
TCDL: 10	Rep Mbr: No	Web: 0.30 (4-8)	Horz TL: 0.09 in		5	
BCLL: 0	Lumber D.O.L.: 125 %					
BCDL: 10						

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	3.5 in	1.50 in	1,265 lbs	.	-52 lbs	-274 lbs	-274 lbs	18 lbs
5	1	3.5 in	1.50 in	1,265 lbs	.	-52 lbs	-274 lbs	-274 lbs	.

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#2 2 x 4

Bracing

TC: Sheathed or Purlins at 3-4-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.
Web: One Midpoint Row: 2-8, 4-8

Loads

- This truss has been designed for the effects of balanced (17.5 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 10 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 115 mph (Factored), Exposure B, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL=1.60
- This truss has been designed for the effects of a 18 psf live load computed in accordance with IBC 2015 assuming slope = 6/12 and area supported = 67.5 ft², DOL = 125 %.
- Minimum storage attic loading has not been applied in accordance with IBC 1607.1
- In accordance with IBC 1607.1, minimum BCLL's do not apply

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.740	432 lbs	(-2,221 lbs)	3-4	0.776	374 lbs	(-1,505 lbs)				
	2-3	0.776	374 lbs	(-1,505 lbs)	4-5	0.740	432 lbs	(-2,221 lbs)				
BC	5-6	0.642	1,913 lbs		6-8	0.623	1,913 lbs		8-9	0.623	1,913 lbs	
Web	2-8	0.300		(-803 lbs)	4-8	0.300		(-803 lbs)				
	3-8	0.197	813 lbs									

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq=0.80).
- Brace bottom chord with approved sheathing or purlins per Bracing Summary
- Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- ↑ ↓ indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSH-B3 for additional information.
- Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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