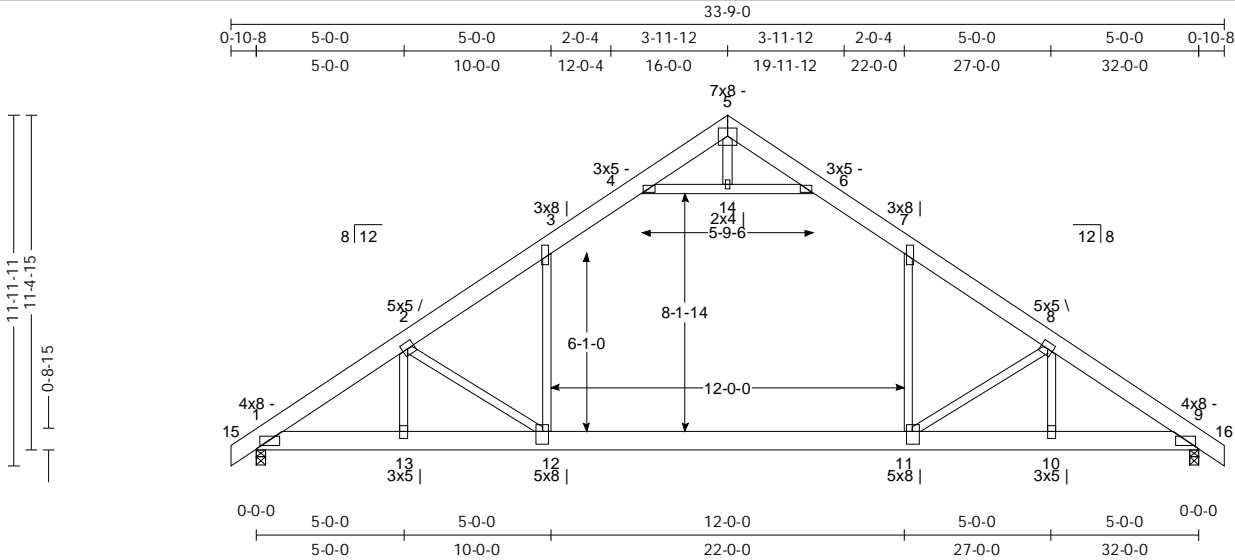


CMT Components

8319 Ashridge Arnhim
Address 2
Sardinia, OH 45171

Truss: Attic
Job: D14aWalker
Designer:CN
Date: 04/14/26 07:49:17
Page: 1 of 2

SPAN 32-0-0	PITCH 8/12	QTY 1	OHL 0-10-8	OHR 0-10-8	CANTL 0-0-0	CANTR 0-0-0	PLYS 1	SPACING 24in	WGT/PLY 264 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2015/	TC: 0.67 (5-6)	Vert TL: 0.74 in	L/512	(11-12)	L/360
GSL: 25	TPI 1-2014	BC: 1.06 (10-11)	Vert LL: 0.37 in	L/999	(11-12)	L/480
TCDL: 10	Rep Mbr: No	Web: 0.54 (6-14)	Horz TL: 0.05 in		9	
BCLL: 0	Lumber D.O.L.: 125 %					
BCDL: 10						

TRUSS DESIGN FAILURE!
MATERIAL DESIGN FAILED
(Material overstressed)

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	3.5 in	2.29 in	1,937 lbs	66 lbs
9	1	3.5 in	2.29 in	1,937 lbs

Material

TC: SYP 2400/2.0 2 x 8
BC: SYP#1 2 x 8
Web: SYP#2 2 x 4

Bracing

TC: Sheathed or Purlins at 5-4-0, Purlin design by Others.
BC: Sheathed

Loads

- This truss has been designed for the effects of balanced (15.9 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 10 with the following user defined input: 25 psf GSL, Terrain B, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 115 mph (Factored), Exposure B, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- This truss has been designed for the effects of a 16 psf live load computed in accordance with IBC 2015 assuming slope = 8/12 and area supported = 67.5 ft², DOL = 125 %.
- Minimum storage attic loading has not been applied in accordance with IBC 1607.1
- In accordance with IBC 1607.1, minimum BCLLs do not apply

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	CSI	Tension (lbs)	Compression (lbs)
TC 1-2	0.152	(-3,053 lbs)	381 lbs
TC 2-3	0.536	(-2,910 lbs)	381 lbs
TC 3-4	0.606	(-1,996 lbs)	381 lbs
BC 9-10	0.465	2,541 lbs	
BC 10-11	1.060	2,541 lbs	
Web 2-13	0.071	(-383 lbs)	1,396 lbs
Web 2-12	0.322	(-572 lbs)	556 lbs
Web 3-12	0.342	1,396 lbs	(-2,780 lbs)

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Attic floor area has been designed as a living area with 40 psf floor live load and a 10 psf floor dead load, and the interior vertical webs and ceiling has been designed for a 5 psf dead load.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Brace bottom chord with approved sheathing or purlins per Bracing Summary
- At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of each web panel point.
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild® Truss Software v5.7.21
Eagle Metal Products

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SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
32-0-0	8/12	1	0-10-8	0-10-8	0-0-0	0-0-0	1	24in	264 lbs

- 8) Listed wind uplift reactions based on MWFRS & C&C loading.
- 9) Bottom chord in the open area of this truss meets L/480 for live load and L/360 for total load deflection criteria.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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